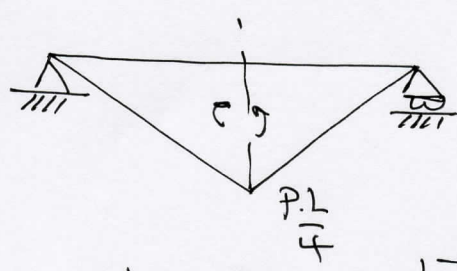
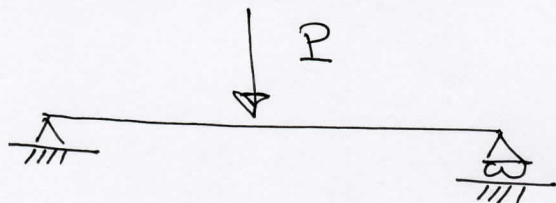


Mecanismos de colapso.

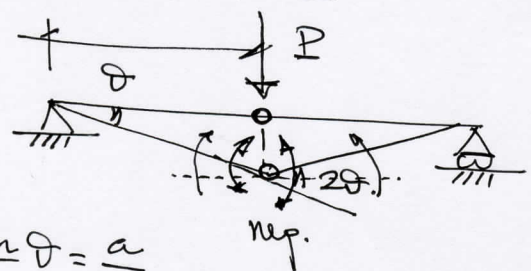
hipótesis simplificadoras:

- 1.- El diagrama de momento-rotación sea bilineal con la rama elástica recta hasta M_p y la rama elastopástica horizontal
- 2.- Despreciamos la influencia de los efectos del arco y del centrado frente al efecto de flexión.



$$M_p = \frac{P \cdot L}{4} \rightarrow \boxed{P = \frac{4M_p}{L}}$$

Principio de los trabajos virtuales:



$$\delta a = a \cdot \delta \theta$$

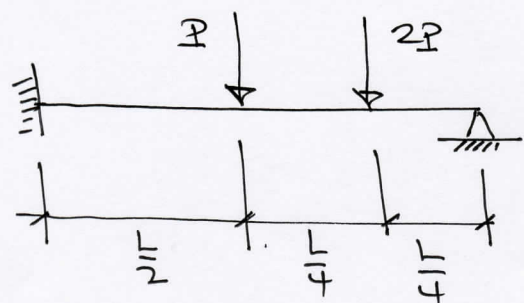
$$W_1 = P \cdot a = P \cdot \delta \cdot \frac{L}{2}$$

$$W_2 = -M_p \cdot 2\delta$$

$$(P \cdot \delta \cdot \frac{L}{2}) - M_p \cdot 2\delta = 0$$

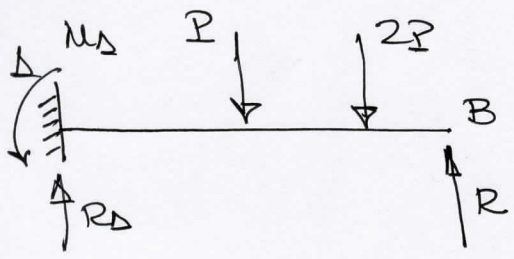
$$\boxed{P = \frac{4M_p}{L}}$$

$$\boxed{P \cdot a = M_p \cdot 2\delta}$$



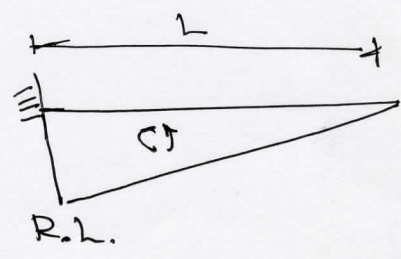
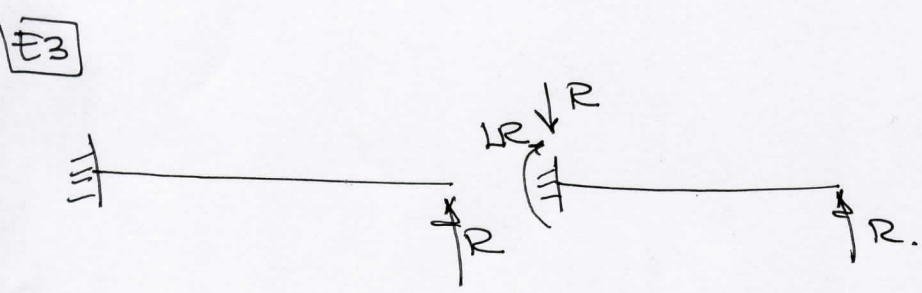
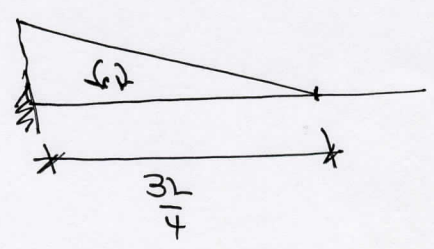
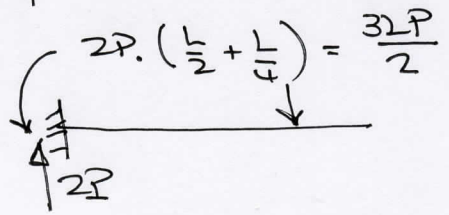
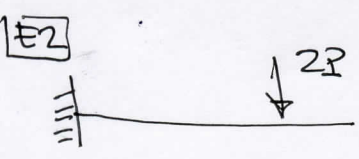
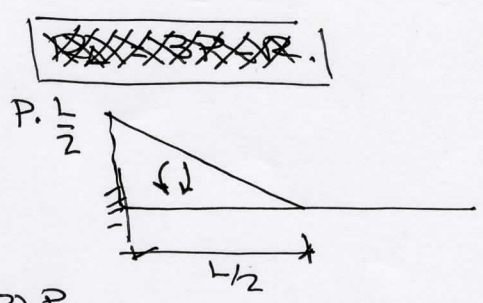
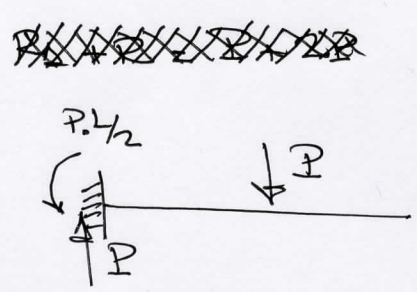
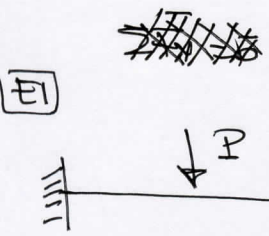
Para fazer a estrutura e após seu deslocamento todas as rótulas com eixos hiperestático mais um.

$$G.H.f = 3 - 2 = 1.$$



$$\boxed{V_B = 0}$$

$$V_B = \frac{V_A}{5} + \frac{\theta_A \cdot L}{5} + \int_A^B \frac{1}{5} \cdot \theta \cdot dS \cdot dS.$$

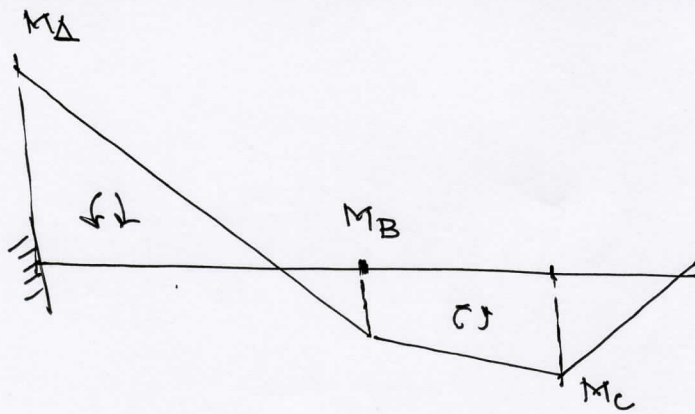


$$0 = \frac{1}{EI} \cdot \left[\frac{1}{2} \cdot P \cdot \frac{L}{2} \cdot \left(\frac{2}{3} \cdot \frac{L}{2} + \frac{L}{2} \right) + \frac{1}{2} \cdot \frac{3L}{4} \cdot \frac{3L}{2} \cdot \left(\frac{L}{2} + \frac{2}{3} \cdot \frac{3L}{4} \right) - \frac{1}{2} \cdot R \cdot L \cdot \frac{2}{3} \cdot L \right].$$

$$\boxed{R = \frac{101}{64} P}$$

$$\sum F_v = 0 \quad R_D + R = P + 2P$$

$$\boxed{R_D = \frac{91}{64} P}$$

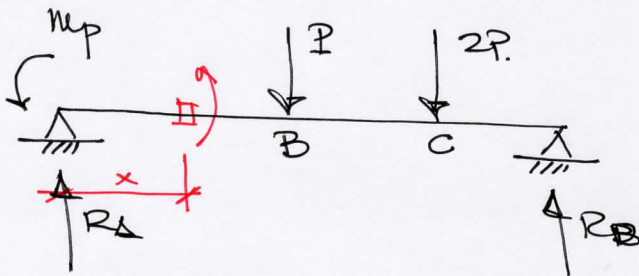


$$M_A = \frac{27}{64} P.L = 0.42 P.L.$$

$$M_B = \frac{37}{128} P.L = 0.28 P.L$$

$$M_C = \frac{101}{256} P.L = 0.39 P.L$$

La puerza rotula se pua en el puto donde el momento flecto es máximo.



$$\sum \bar{F}_V = 0 \quad R_A + R_B = 3P \quad (1)$$

$$\sum \bar{M}_A = 0 \quad M_p + R_B \cdot L - P \cdot \frac{L}{2} - 2P \cdot \frac{3L}{4} = 0 \quad (2)$$

$$R_A = 3P - [2P - \frac{M_p}{L}] = P + \frac{M_p}{L}$$

$$R_B = [2P - \frac{M_p}{L}]$$

1. SOSTA'NICA.

TRANSAB.

$$\sum \bar{M} = 0 \quad \bar{M}(x) + M_p - R_A x = 0$$

$$\bar{M}(x) = R_A x - M_p$$

$$\text{en B } x = \frac{L}{2}$$

$$\bar{M}(x) = R_A \cdot \frac{L}{2} - M_p =$$

$$= (P + \frac{M_p}{L}) \cdot \frac{L}{2} - M_p =$$

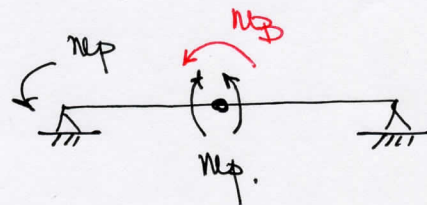
$$= P \cdot \frac{L}{2} + \frac{M_p}{2} - M_p =$$

$$= (P \cdot \frac{L}{2} - \frac{M_p}{2})$$

$$M_B = M_p - R_A \cdot \frac{L}{2} = M_p - (P + \frac{M_p}{L}) \cdot \frac{L}{2} = (\frac{M_p}{2} - \frac{P \cdot L}{2})$$

$$M_C = R_B \cdot \frac{L}{4} = (2P - \frac{M_p}{L}) \cdot \frac{L}{4} = (\frac{P \cdot L}{2} - \frac{M_p}{4})$$

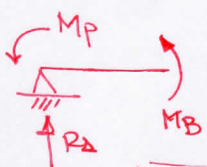
Supuamos fe adapra en B:



$$M_p = (-\frac{M_p}{2} + \frac{P \cdot L}{2})$$

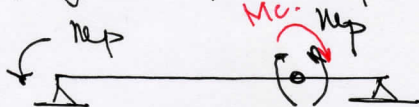
$$2M_p = -M_p + P \cdot L$$

$$P = \frac{3M_p}{L}$$



$$M_p = -M_B$$

Supuamos fe colapra en C:

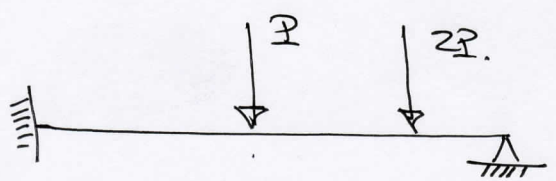


$$M_p = \frac{P \cdot L}{2} - \frac{M_p}{4}$$

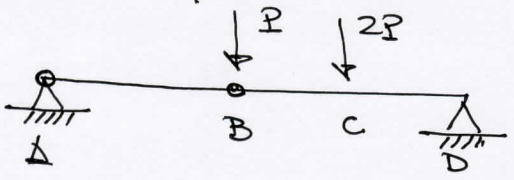
$$M_p = M_C$$

$$P = \frac{5}{2} \frac{M_p}{L}$$

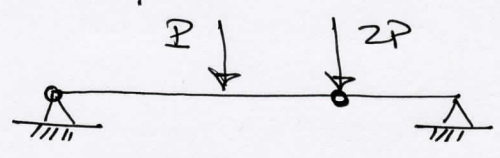
$GH = 1.$ $n^{\circ} \text{ Roturas} = 2$



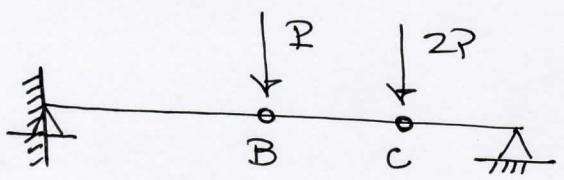
I ΣM_A y ΣM_B .



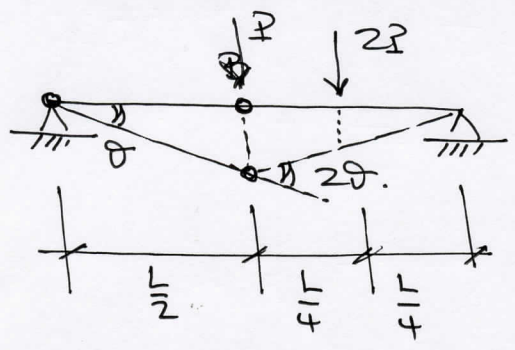
II ΣM_A y ΣM_C .



III ΣM_B y ΣM_C .



I



$$(Pa + 2P \cdot b) = M_p (\theta + 2\theta)$$

$$\tan \theta = \frac{a}{L/2} = \frac{2a}{L} \longrightarrow \theta = \frac{2a}{L} \quad \boxed{a = \frac{1}{2} \theta \cdot L}$$

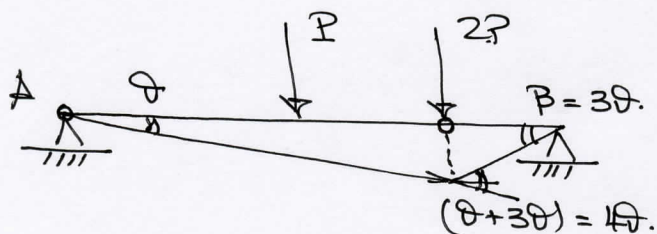
$$\tan \theta = \frac{b}{L/4} = \frac{4b}{L} \longrightarrow \theta = \frac{4b}{L} \quad \boxed{b = \frac{1}{4} \theta \cdot L}$$

$$\left(P \cdot \frac{1}{2} \theta + 2 \cdot P \cdot \frac{1}{4} \theta \right) = M_p (\theta + 2\theta)$$

$$\boxed{P = \frac{3}{L} M_p}$$

II Ex 4 y c.

5



$$\tan \theta = \frac{a}{L/2} = \frac{b}{3L/4}$$

$$b = \frac{3L}{4} \cdot \theta$$

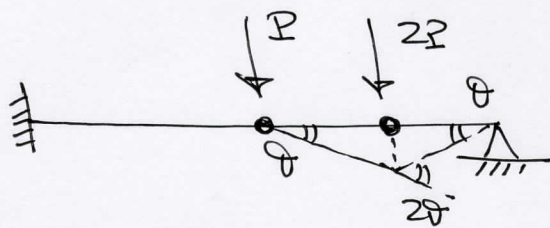
$$\tan \theta = \frac{b}{L/4} = \frac{3L/4 \cdot \theta}{L/4} = 3\theta$$

$$(P \cdot a + 2P \cdot b) = M_p \cdot (\theta + 4\theta)$$

$$\left(P \cdot \frac{L}{2} \theta + 2P \cdot \theta \cdot \frac{3L}{4} \right) = M_p (\theta + 4\theta)$$

$$\boxed{P = \frac{5}{2} \frac{M_p}{L}}$$

III Ex 3 y c.



$$\tan \theta = \frac{a}{L/4} \quad a = \theta \cdot \frac{L}{4}$$

$$2P \cdot a = M_p \cdot 2\theta$$

$$2P \cdot \theta \cdot \frac{L}{4} = M_p \cdot 2\theta$$

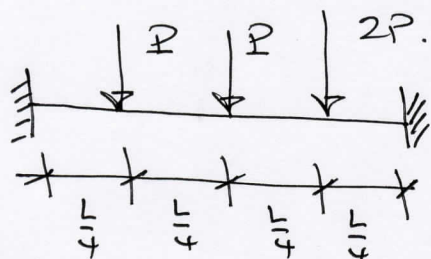
$$\boxed{P = \frac{6M_p}{L}}$$

La soluci3 es la fe rep3er una cosa m3s \rightarrow Ex 2 II

$$\boxed{P = \frac{5}{2} \frac{M_p}{L}}$$

Una viga de horm de luz de sección constante de anchura "a" y canto "2a" está sujeta a las cargas p.e. indicadas.

- Se pide:
- 1) Dimensionar la viga para la tensión plástica.
 - 2) Mecanismos de colapso.
 - 3) Reacciones y momentos flectores en el instante de colapso.



$$G.H = 4 - 2 = 2$$

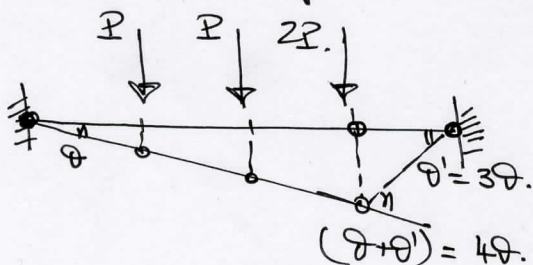
$$n^{\circ} \text{ rótulas} = 3$$

$$P = 160 \text{ kN}$$

$$q = 10^5 \text{ kN/m}^2$$

$$n^{\circ} \text{ de mecanismos posibles: } \binom{5}{3} = 10$$

De los 10 mecanismos posibles hemos de seleccionar aquellos en los que las rótulas se produzcan lo más próximo posible a las zonas extremas.



$$L\theta = \frac{a}{\frac{3}{4}L}$$

$$a = \frac{3}{4}L\theta$$

$$L\theta' = \frac{a}{\frac{1}{4}L}$$

$$a = \frac{1}{4}L\theta'$$

$$(\theta + \theta') = 4\theta$$

$$\frac{a}{\frac{3}{4}L} = \frac{b}{\frac{1}{2}L} = \frac{c}{\frac{1}{4}L} = L\theta$$

$$\frac{3}{4}L\theta = \frac{1}{4}L\theta'$$

$$\boxed{\theta' = 3\theta}$$

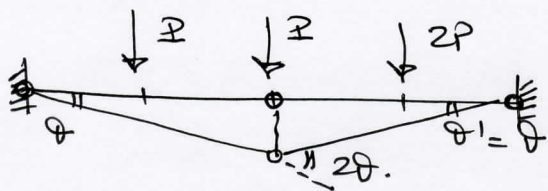
$$b = \frac{1}{2}L\theta$$

$$c = \frac{1}{4}L\theta$$

$$(P \cdot \frac{1}{4}L\theta + P \cdot \frac{1}{2}L\theta + 2P \cdot \frac{3}{4}L\theta) = M_p \cdot \theta + M_p \cdot 4\theta + M_p \cdot 3\theta$$

$$P \left[\frac{1}{4} + \frac{1}{2} + \frac{3}{2} \right] = 8M_p$$

$$\boxed{P} = \frac{8M_p}{\frac{9L}{4}} = \frac{32M_p}{9L}$$



$$\theta = \theta'$$

$$\frac{1}{2}\theta = \frac{a}{L/2} = \frac{b}{L/4}$$

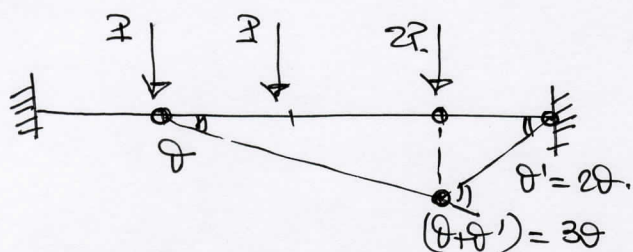
$$a = \frac{1}{2}\theta$$

$$b = \frac{L}{4}\theta$$

$$\left(P \cdot \frac{L}{4}\theta + P \cdot \frac{L}{2}\theta + 2P \cdot \frac{L}{4}\theta \right) = M_p \cdot \theta + M_p \cdot 2\theta + M_p \cdot \theta$$

$$P \left(\frac{L}{4} + \frac{L}{2} + \frac{L}{2} \right) = 4M_p$$

$$\boxed{P = \frac{4M_p}{\frac{5}{4}L} = \frac{16M_p}{5L}}$$



$$\frac{1}{2}\theta = \frac{a}{L/2}$$

$$\frac{1}{2}\theta = \frac{L}{4}\theta'$$

$$\boxed{\theta' = 2\theta}$$

$$\frac{1}{2}\theta' = \frac{a}{L/4}$$

$$\frac{1}{2}\theta = \frac{a}{L/2} = \frac{b}{L/4}$$

$$P \frac{L}{4}\theta + 2P \cdot \frac{L}{2}\theta = M_p \cdot \theta + M_p 3\theta + M_p 2\theta$$

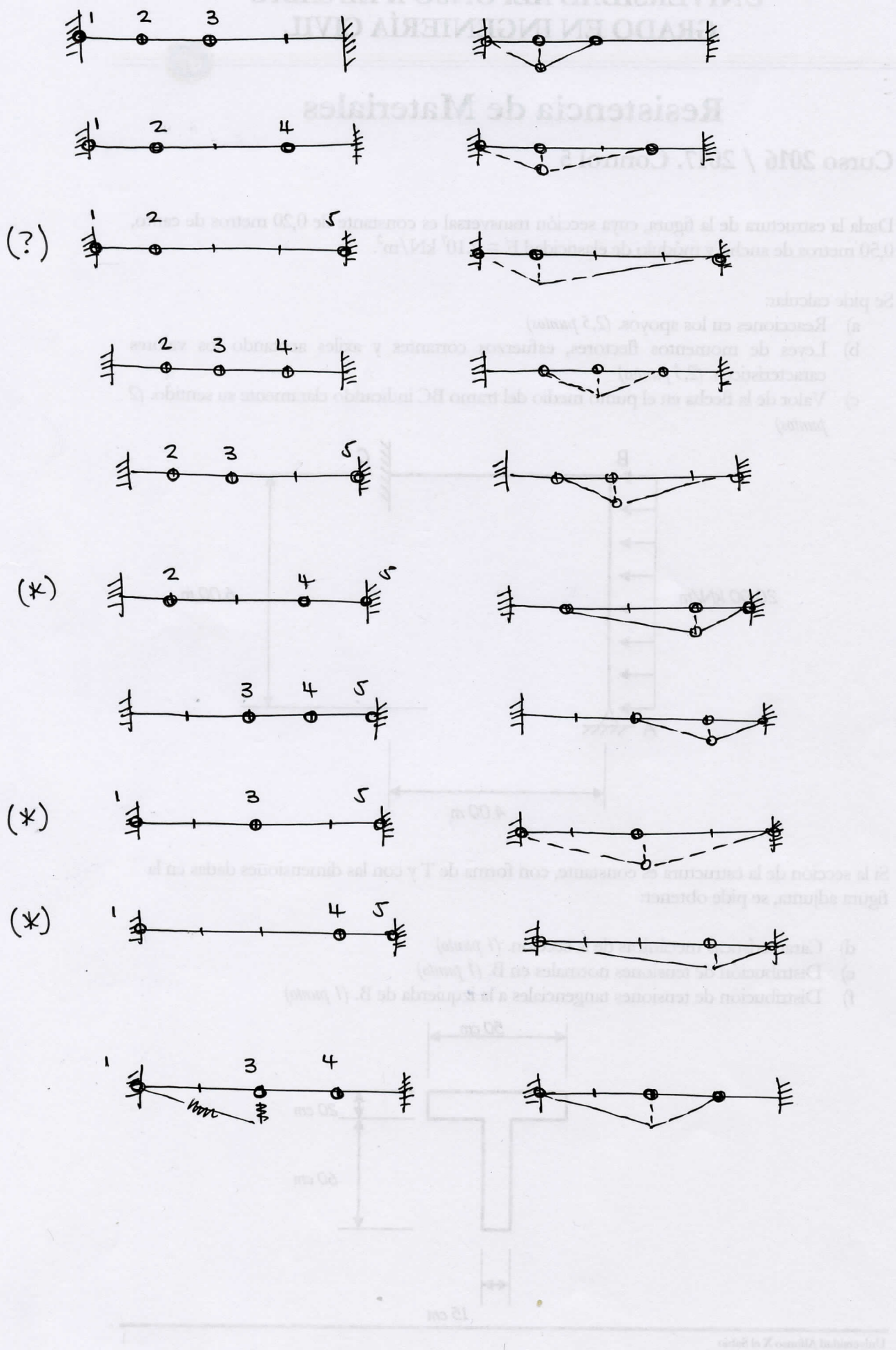
$$P \left(\frac{L}{4} + 2 \cdot \frac{L}{2} \right) = 6M_p$$

$$\boxed{P = \frac{6M_p}{\frac{5}{4}L} = \frac{24M_p}{5L}}$$

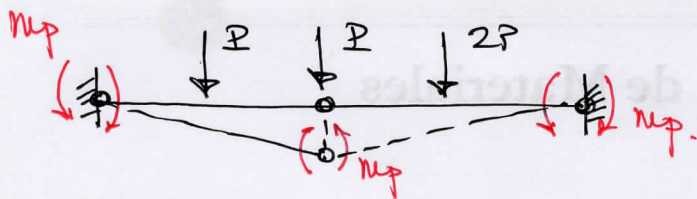
El mecanismo de colapso es el siguiente:

$$\boxed{P = \frac{16M_p}{5L}}$$

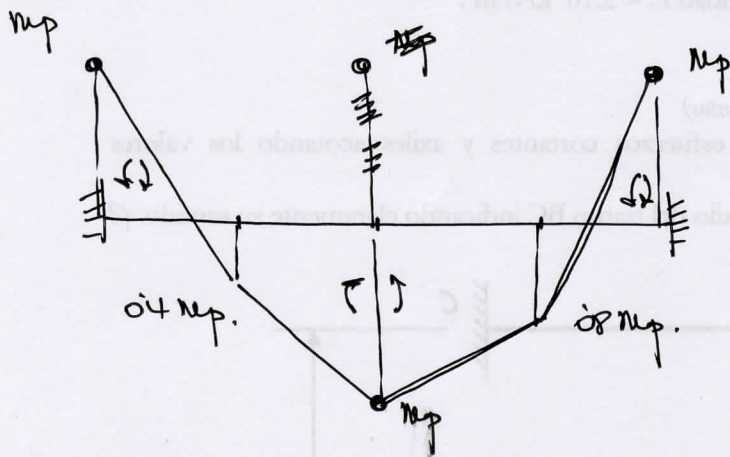
Posibles sucesos de rotura.



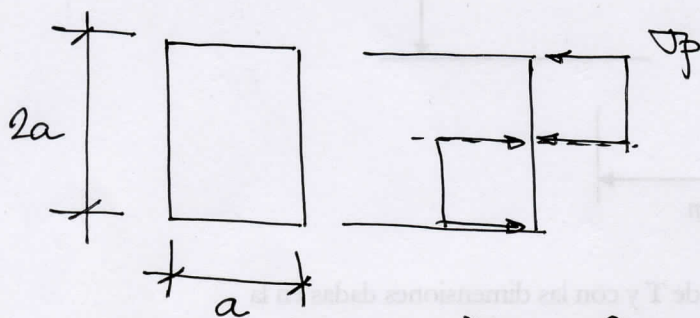
Mecanismos de colapso:



Ley de flexión del mecanismo de colapso:



Momento plástico. (flexión por)



$$M_p = 2\sigma_p \cdot a \cdot a \cdot \frac{a}{2} = a^3 \cdot \sigma_p$$

ley de colapso: $\left| P = \frac{16}{5} \frac{M_p}{L} \right| \longrightarrow M_p = \frac{5}{16} P \cdot L$

$$a^3 \cdot \sigma_p = \frac{5}{16} P \cdot L$$

$$P = 160 \text{ kN}$$

$$\sigma_p = 10^5 \text{ kN/m}^2$$

$$L = 16 \text{ m}$$

$$a^3 \cdot 10^5 = \frac{5}{16} \cdot 160 \cdot 16$$

$$a = \sqrt[3]{\frac{5 \cdot 160 \cdot 16}{16 \cdot 10^5}} = 0.2 \text{ m} = \boxed{20 \text{ cm}}$$

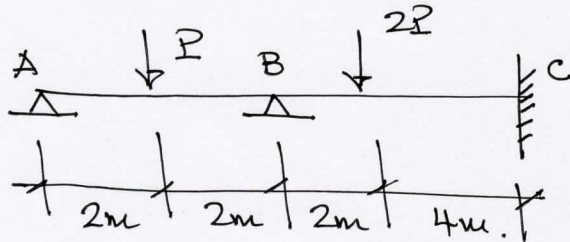
VIGAS CONTINUAS.

Consideraciones:

- 1) Se acepta que la plastificación de un tramo es suficiente para considerar plastificada totalmente la estructura, por lo que la viga continua se estudia tramo a tramo considerando el resto de la viga por un desplazamiento.
- 2) El n° de rótulas así es independiente del número de tramos de la viga.
- 3) Se despreja de el efecto del arañe ya que se supone que la plastificación ~~se alcanza~~ se alcanza sólo por efecto de flexión.
- 4) Si la rigidez fue distinta en cada tramo y se poseen rótulas plásticas en las zonas de unión de tramos contiguos, éstas aparecerán en los tramos de menor rigidez.

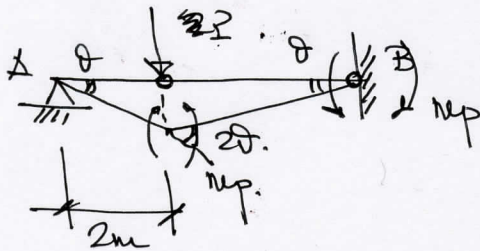
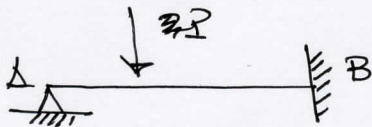
Dada la viga continua de la fig., de sección rectangular $30 \times 50 \text{ cm}$ sujeta a las cargas se indican, se pide hallar:

- 1) La carga de plastificación
- 2) Determinar el mecanismo de rotura
- 3) Ley de momentos flectores del momento de rotura sabiendo que la tensión de fluencia del material es de 15 MPa .



TRANS 1. ΔB

$$n^{\circ} \text{ rotulas} = 2. \quad (GHI = 1)$$



$$\frac{1}{2} \theta \approx \theta = \frac{a}{2} \quad a = 2\theta.$$

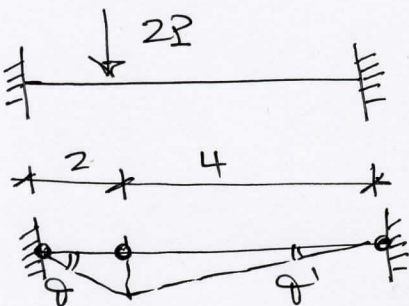
$$(P.a) = 2\theta \cdot Mp + \theta \cdot Mp$$

$$\boxed{P = \frac{3Mp}{2}}$$

$$(\text{Trabajo f. externos}) = (\text{Trabajo rotulas plasticas.})$$

TRANS 2 BC.

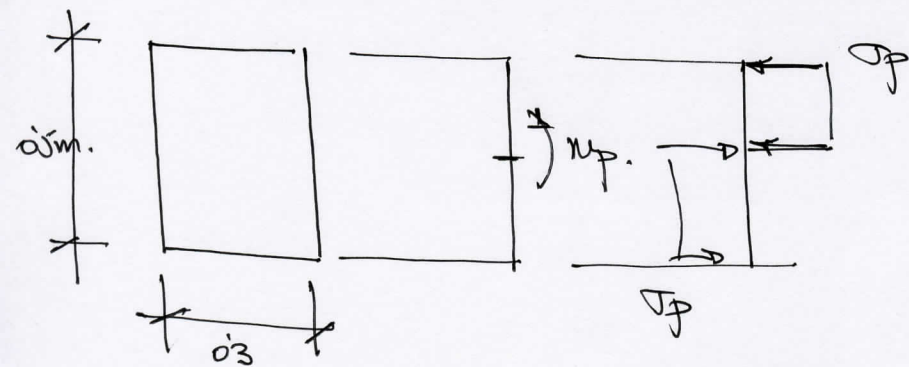
$$GHI = 2. \rightarrow n^{\circ} \text{ rotulas} = \underline{\underline{3}}$$



$$\left. \begin{aligned} \frac{1}{2} \theta &= \frac{a}{2} \\ \frac{1}{2} \theta' &= \frac{a}{4} \end{aligned} \right\} \begin{aligned} 2\theta &= 4\theta' & \theta' &= \frac{\theta}{2} \\ \theta + \theta' &= \theta + \frac{\theta}{2} & &= \frac{3}{2} \theta. \end{aligned}$$

$$2P.a = Mp \cdot \theta + Mp \cdot \frac{3}{2} \theta + Mp \cdot \frac{\theta}{2} \quad (a = 2\theta) \quad \boxed{P = \frac{3Mp}{4}}$$

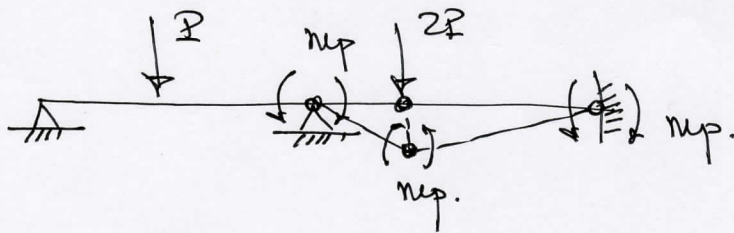
carga de plastificación: $P = \frac{3}{4} M_p$. (la usas figura).



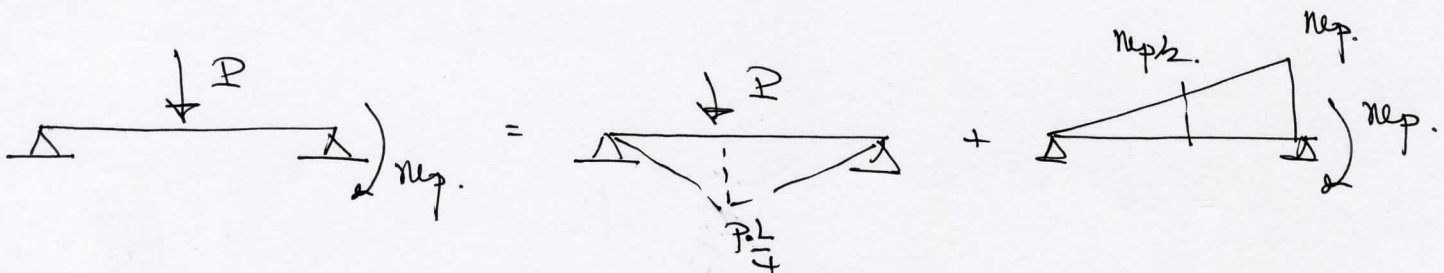
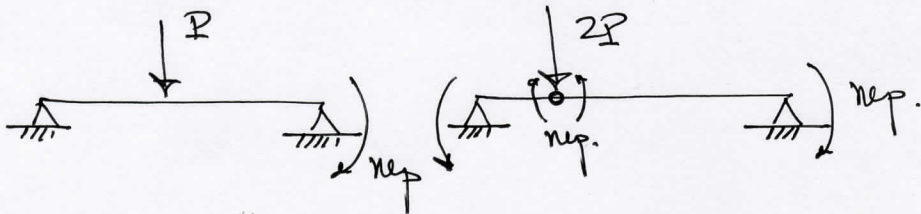
$$M_p = 2\sigma_p \cdot \left[\frac{0.25}{2} \cdot 0.3 \cdot \frac{0.25}{2} \right] = 0.01875 \cdot \sigma_p = \boxed{281.25 \text{ kN.m.}}$$

$$P = \frac{3}{4} M_p = \boxed{210.93 \text{ kN.}}$$
 carga de colapso.

Mecanismos de colapso:



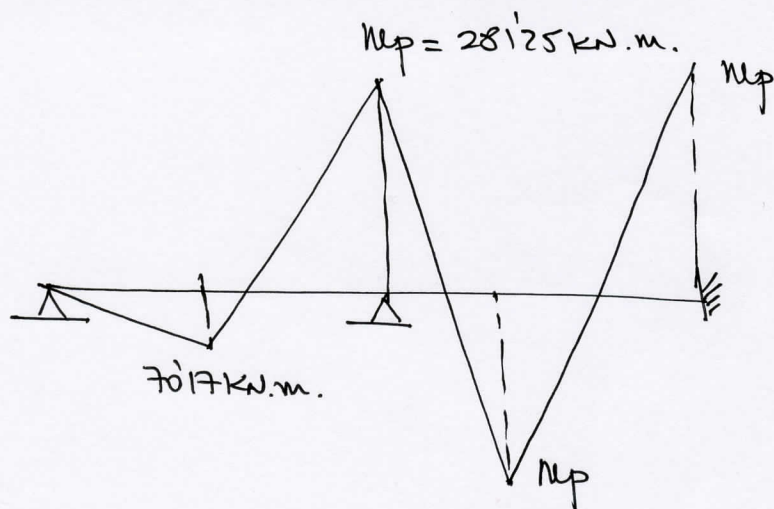
ley de flexión en el colapso:



$$\frac{P \cdot L}{4} = \frac{210.93 \cdot 4}{4} = 210.93 \text{ kN.m.}$$

$$\frac{M_p}{2} = \frac{281.25}{2} = 140.625 \text{ kN.m.}$$

$$\left(\frac{P \cdot L}{4} - \frac{M_p}{2} \right) = 210.93 - 140.625 = \boxed{70.305 \text{ kN.m.}}$$



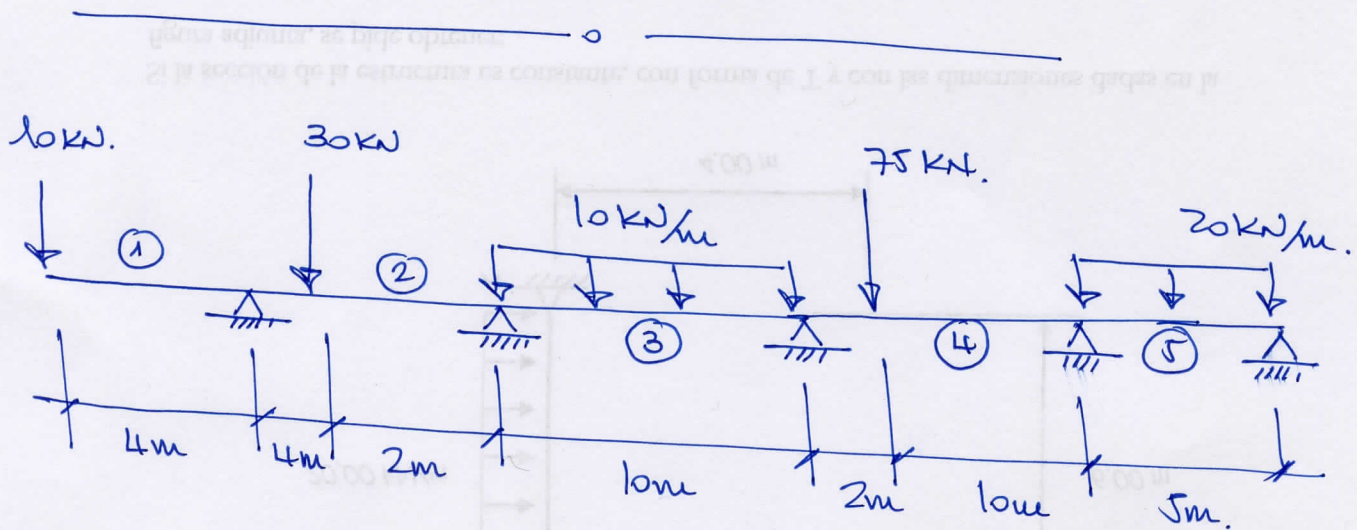
Dada la nra carta de la fpm adjunta de nra veraz
sacada a un usura de agn extors en eflio & fide halle:

- 1) El momento preciso y dinámico la una siendo se re-
trayendo de la el centro y a del centro.
Si caso es ~~en el~~ siendo se ~~en el~~
los como dados producen el apatamiento.

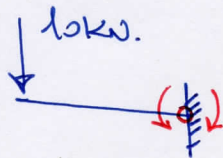
Factor de fura 1/2.

$$\sigma_f = 24 \cdot 10^5 \text{ KJ/m}^2$$

- 2) Mecanismos de colapso.
- 3) Reacciones y ley de momentos flectores en el instante de colapso.



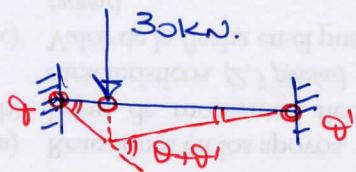
Trans (A).



$$M_p = P.L = 10.4 = \boxed{40 \text{ kN.m.}}$$

Trans (2)

GH 22
N = 3.



$$\lg a = \frac{a}{4}$$

$$a = 49$$

$$\theta' = 4\theta = 2\theta'$$

$$\tan \theta' = \frac{a}{2}$$

$$a = 2\theta'$$

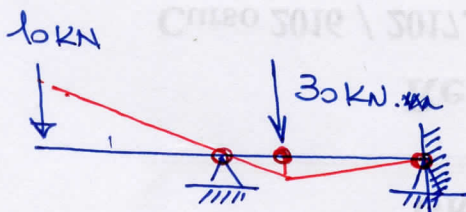
$$\theta' = 2\theta.$$

$$30. a = n_p \theta + n_p.(\theta + \theta') + n_p. \theta'$$

$$30.4\theta = \mu_p \theta + \mu_p 3\theta + \mu_p 2\theta$$

$$120 = 6m_p$$

$$\eta_p = \frac{P_o}{P} = \boxed{20 \text{ kW.m.}}$$



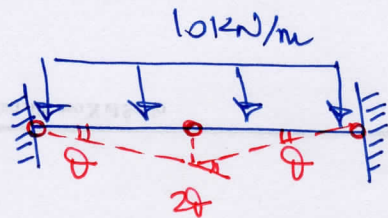
$$\lg \theta = \frac{b}{4}$$

$$b = 49$$

$$(30.4\theta) - 10.4\theta = m_p (\theta + \theta') + m_p \theta'$$

$$(1200 - 400) = \text{mp. } 30 + \text{mp. } 20 \quad 80 = 5 \text{ mp} \quad \boxed{\text{mp} = 16\%}$$

TRAMO 3.



$$\tan \theta = \frac{a}{5}$$

$$a = 5\theta$$

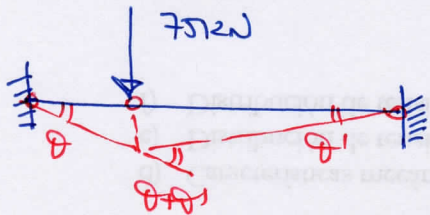
$$\Delta BED = \frac{1}{2} 5\theta \cdot 10 = 25\theta$$

$$(10 \cdot \frac{1}{2}) 25\theta = M_p \cdot \theta + M_p 2\theta + M_p \theta$$

$$250\theta = 4 M_p$$

$$M_p = \frac{250\theta}{4} = \boxed{62.5 \text{ kN.m.}}$$

TRAMO 4



$$\tan \theta = \frac{a}{2}$$

$$a = 2\theta$$

$$\tan \theta' = \frac{a}{10}$$

$$a = 10\theta'$$

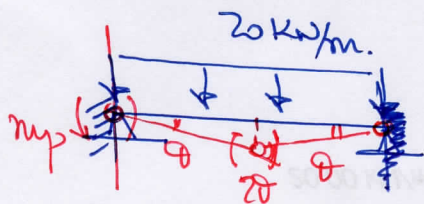
$$2\theta = 10\theta'$$

$$\theta' = \frac{1}{5}\theta$$

$$(75 \cdot 2\theta) = M_p \cdot \theta + M_p (\theta + \frac{1}{5}\theta) + M_p \cdot \frac{1}{5}\theta$$

$$750 = 5M_p + 6M_p + M_p \quad M_p = \frac{750}{12} = \boxed{62.5 \text{ kN.m.}}$$

TRAMO 5.



$$\tan \theta = \frac{a}{2.5}$$

$$a = 2.5\theta$$

$$\Delta BED = \frac{1}{2} 2.5\theta \cdot 5 = 6.25\theta$$

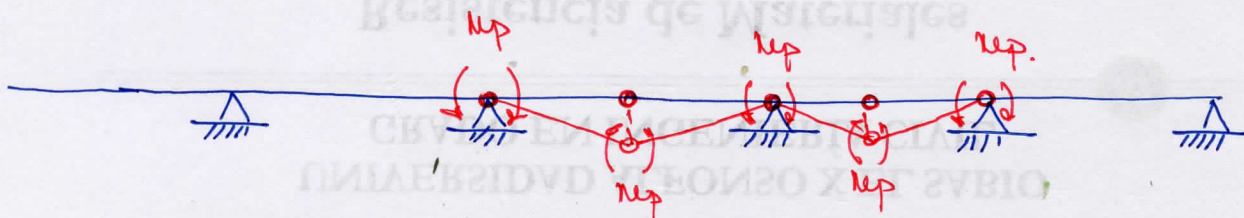
$$(20 \cdot 6.25\theta) = M_p \theta + M_p 2\theta$$

$$M_p = 41.6 \text{ kN.m.}$$

$$M_p = 62.5 \text{ kN.m.}$$

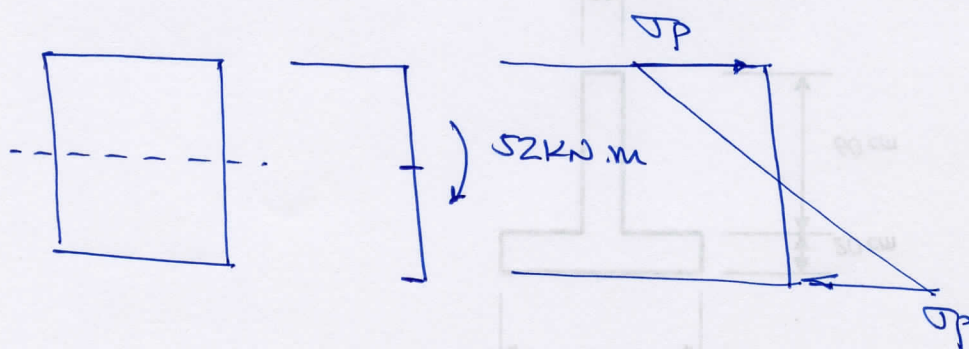
El momento plástico es el mayor de los necesarios por la viga entre los apoyos en cada tramo.

MECANISMO DE ROTURA:



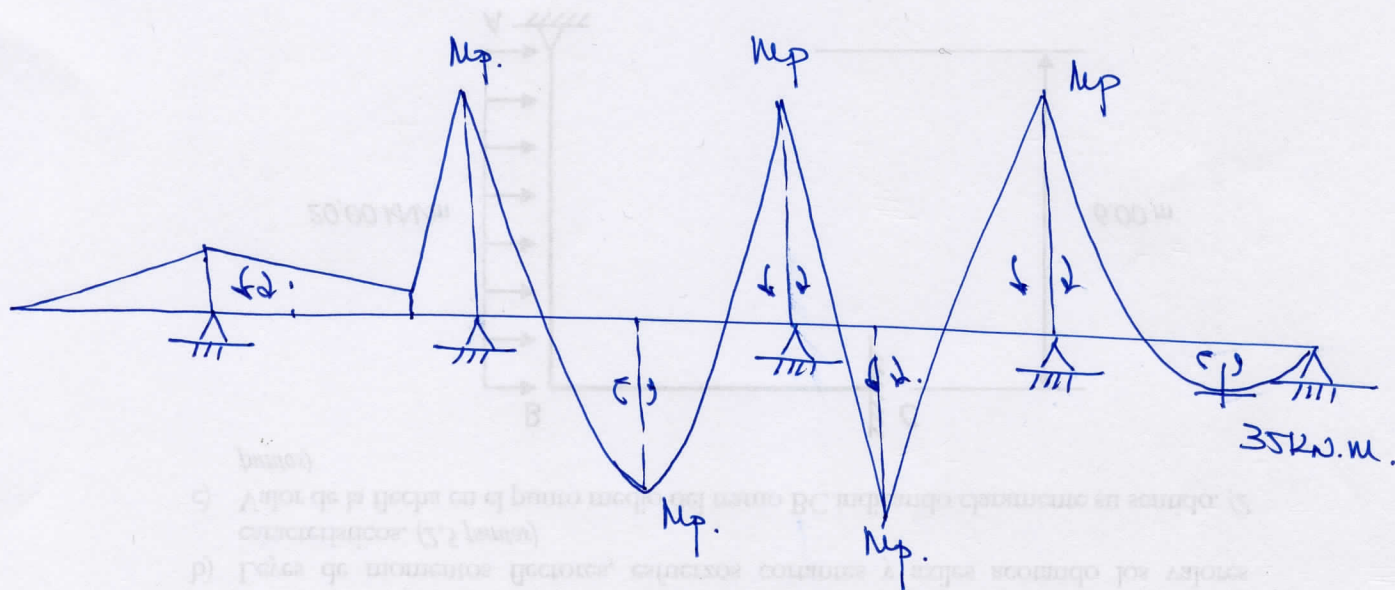
$$M_p = \boxed{625 \text{ KN.m}}$$

$$M_e = \frac{M_p}{\lambda} = \frac{625}{12} = \boxed{52 \text{ KN.m}}$$



$$\sigma_p = \frac{M_e \cdot y}{I}$$

LEY FLECHURAL



Curso 2016 / 2017 Control 2

Resistencia de Materiales

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UNIVERSIDAD TECNICA X ET SABIO